



Analytical Study on Reduced Beam Section Moment Connections Affecting Cyclic Behavior

R.Rahnavard¹, J.Delfian², H.Poursardar³

M.Sc Student of Structure Engineering, Dep. of Civil Engineering, ACECR of Ahvaz, Iran.
M.Sc Student of Structure Engineering, Dep. of Civil Engineering, Azad university of Dezfol, Iran.
M.Sc Student of Earthquake Engineering, , Research Institude of ShakhesPajouh, Esfahan, Iran.

Abstract

The recent earthquakes have shown that steel moment frame (SMF) with weld connections are so brittle. According to studied, the great damages are due to cracking of the weld between the beam flange and the column face and inducing concentrated stresses in this area. A useful approach to reduce the stress concentration at the panel zone could be using of reduced beam section (RBS). Given the enormous impact of seismic behavior and ductility of the panel zone, RBS moves plastic hinge formation at an appropriate distance from column face. In this study, an ordinary rigid connection, reduce beam flange section and reduce beam web section have been modeled using ABAQUS computer program and compared with each other during cyclic behavior. The obtained result of this study showed that reduced beam flange section is more ductile and will dissipate energy more than other connections.

Keywords: Dissipated energy, Plastic hinge, RBS connections, Stiffness classification.

1. Introduction

This Since 1994 Northridge earthquake, many studies have been performed to replace better connections for new steel moment frame and to enrich poor moment connections for exiting steel moment frames. The prior and post-Northridge laboratory observations have also demonstrated the inherent disability of the conventional moment connections to develop enough ductility (Engelhard and Husain 1993, Calado 2000)[1].

Since the Northridge earthquake, a number of various studies have been carried out in order to improve the seismic performance of the conventional welded connections. One of the most promising ways to modify the behavior of the conventional moment frame is to soften a portion of beam flanges near the column face (Plumier 1997, Engelhard et al. 1998, Yu et al. 1999, Yu and Yang 2001)[2]. The connections softening may be accomplished by trimming circular selectors from the beam flanges near the column. This solution, so-called reduced beam section (RBS) method, leads plastic hinges toward the beam span away from column face, resulting in reduction of stress concentration at the interface of beam and column. However, as the result of reducing the beam section within a sensitive zone, the beam becomes more prone to buckling. Some studies have been conducted to assess key issues influencing the instability of RBS beams (Yang and Fan 1999, Yang and Fan 2001, Moslehi Tabar and Deylami 2005)[3]. Moslehi Tabar and Devlami (2005)[3] defined a new lateral slenderness parameter, which is in good agreement with the experimental data. According to this definition, the cyclic behavior of RBS beams is mostly affected by their lateral instability and the beam depth-to-length ratio. Column panel zone flexibility is another concern affecting the behavior of RBS connections. Tsai and Chen (2002) and Jones et al. (2002) experimentally illustrated that RBS moment connections with moderately strong panel zones show appropriate performance. Sang-Whan Han, Ki-Hoon Moon and Bozidar Stojadinovic (2009)(2010)[4,5] studied the design equations of RBS connections and found that RBS-B connection moment strength equation specified in FEMA-350, consistently overestimates the actual strength of the RBS-B connections measured in tests and the reduction of beam sections according to FEMA-350 which maybe therefore insufficient to protect the RBS-B connections. This, in turn, may lead to RBS-B connection failure before a plastic hinge forms at the reduced beam section of the beam. Rahnavard and Siahpolo (2013)[6] have studied bolt and weld moment connections in both with and without reduced section. They made some models in Abaqus software and compared them to find that the RBS connections increase ductility of beam and panel zone and also will result in reduction of stress and plastic strain concentration at the interface of beam and column. Moslehi Tabar and Devlami (2013)[7] considered a new detail and proposed to enhance the RBS performance by delaying beam buckling. The efficiency of the proposed detail was investigated by a large scale laboratory testing under cyclic loading. The results of their study showed that the proposed RBS connection had superior performance as compared with the conventional one. The proposed detail increased plastic rotation capacity of the conventional RBS connection.